

1 **CLOSURE: CASE STUDY: OSO, Washington, Landslide**  
2 **of 22 March 2014: Dynamic Analysis**

3 Jordan Aaron

4 Postdoc, Engineering Geology

5 ETH Zürich

6 Sonneggstrasse 5

7 8048 Zürich, Switzerland

8 phone: +41 (44) 633-8072

9 email: jordan.aaron@erdw.ethz.ch

10  
11 and

12  
13 Timothy D. Stark and Ahmed K. Baghdady

14 Professor and Graduate Research Assistant of Civil and Environmental Engineering

15 University of Illinois at Urbana-Champaign

16 205 N. Mathews Ave.

17 Urbana, IL 61801

18 (217) 333-7394

19 (217) 333-9464 Fax

20 email: tstark@illinois.edu and baghdad2@illinois.edu

21

# CLOSURE:

## CASE STUDY: OSO, Washington, Landslide of 22 March 2014: Dynamic Analysis

Jordan Aaron, Timothy D. Stark and Ahmed K. Baghdady

The writers appreciate the thorough discussion provided by Richard M. Iverson, which raises some important points concerning our dynamic analysis of the Oso flowslide. This closure is structured as follows: (1) response to Dr. Iverson's criticisms of our runout analysis methodology, (2) clarification of comments in Aaron et al. (2017) regarding the modelling results of Iverson et al. (2015) and Iverson & George (2016), (3) comparison of the simulations of Iverson et al. (2015) and Iverson & George (2016) with those of Aaron et al. (2017) in light of the present discussion, and (4) summary of our runout analysis.

### Criticisms of Runout Analysis in Aaron et al. (2017)

Dr. Iverson states the central scientific problem of explaining the mobility of the Oso flowslide is quantifying the cause-and-effect mechanism by which the landslide dynamics and large undrained strength, i.e., liquefaction, are coupled. This criterion is then used to judge our analysis of the Oso flowslide. Dr. Iverson claims that, because the initial conditions of our model do not correspond to static equilibrium, our modelling results provide no basis to understand the mechanisms acting during the Oso flowslide. Dr. Iverson provides two sets of analyses to demonstrate that our model does not begin from a state of static equilibrium. The first analysis uses a comparison of our average effective friction coefficient to that required for static equilibrium, and shows that our strengths, derived from inverse runout analysis, are

45 too low for static equilibrium. The second analysis uses an energy balance to show that our  
46 inverse analysis derived strengths imply that the landslide commenced with a large,  
47 instantaneous release of energy.

48 We fundamentally disagree with the criteria Dr. Iverson has used to judge our analysis. As is  
49 elaborated more fully below, we do not think that runout models must start from a statically  
50 balanced initial state to provide useful results and the liquefied strength was not operating at  
51 static equilibrium. It has long been recognized that, due to the complexity of earth materials, a  
52 meaningful geotechnical analysis must balance accurate site characterization with careful use  
53 of idealized/conceptual models, all of which must be moderated with judgement (Burland,  
54 1987; Goodman, 1999; Hungr, 2016). The criteria and criticisms presented by Dr. Iverson  
55 are heavily weighted towards formulating idealized models, and this comes at the expense of  
56 recognizing the complexity of earth materials, and the judgement required to interpret results  
57 from idealized models in a meaningful way. As will be discussed in the following sections,  
58 our analysis has attempted to balance accurate site characterization with careful use of two  
59 semi-empirical runout models, to perform a meaningful dynamic analysis of the Oso  
60 Landslide that is in good agreement with field observations.

61 Dr. Iverson misunderstood the *Analysis of Landslide Mechanism* section presented in Aaron et  
62 al. (2017). This is possibly due to the use of different definitions of the word ‘failure’. In  
63 Aaron et al. (2017), as well as the present discussion, we use the Hungr et al. (2014)  
64 definition, which defines failure as the “single most significant movement episode in the  
65 known or anticipated history of the landslide...”(Hungr et al., 2014, p. 167). The shear  
66 strength parameters that we derived through our inverse runout analysis correspond to the  
67 residual strength acting during the flowslide, discussed below. We never claimed that our  
68 model begins from a statically balanced initial state nor do we believe that it does. However,

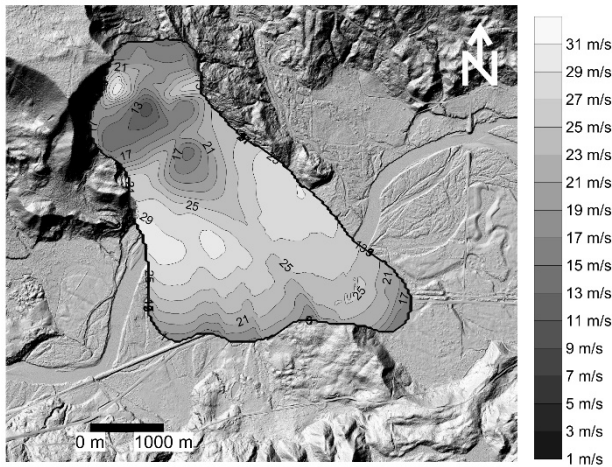
69 as summarized below, we think that our model can be used to investigate certain questions  
70 relating to the runout of the Oso flowslide.

71

72 Dr. Iverson's comparison of our initial conditions to a slingshot and levitation are misleading,  
73 as we do not introduce an artificial source of energy because our failure mechanism involves  
74 the upper portion of the slope that has a high potential energy. The initial conditions of our  
75 model correspond to the point in time when the shear strength on the rupture surface in the  
76 water-filled colluvium has been reduced to its minimum or residual value. The initial force  
77 imbalance is due to brittle failure of the upper portion of the slope. The landslide accelerates  
78 from the at-rest condition due to gravity and internal pressure gradients (as summarized in the  
79 equations of motion presented in Hungr (1995) and McDougall & Hungr(2004)) and it  
80 impacts the water-filled colluvium.

81 Our implicit simplification is that this undrained strength loss in the colluvium is  
82 instantaneous; a simplification that we believe is justified because we focus only on  
83 simulating the runout of the flowslide. As shown on Figure 1, our maximum simulated  
84 velocities are similar to the average velocities reported by Iverson & George (2016, p. 181),  
85 suggesting that this simplification is justified. This is a well-known and commonly used  
86 simplification, and users of semi-empirical runout models are familiar with and acknowledge  
87 it. Through careful inverse analysis, models that use this simplification have been a useful  
88 tool to investigate certain mechanisms acting during the motion of extremely-rapid, flowlike  
89 landslides (e.g. Hungr et al., 2002; Hungr & Evans, 2004; McDougall et al., 2006; Sosio et al.,  
90 2008; Aaron & Hungr, 2016; Hungr, 2017). Of course, our model cannot answer the question  
91 "Will a large undrained strength loss occur?", instead it provides insights into the question  
92 "Given that a large undrained strength loss has occurred, what is the expected final deposit

93 distribution?”. We have only interpreted our model results to answer the second question, and  
94 therefore believe that our conclusions are valid because they match field conditions.



95

96 *Figure 1: Maximum simulated velocities at each grid node in the computational domain*  
97 *for the Phase A simulation in Aaron et al. (2018).*

98

99 We parameterize the liquefied strength by using a liquefied strength ratio (Olson & Stark,  
100 2002), an approach that is criticized by Dr. Iverson. Stark & Mesri (1992) introduced the use  
101 of a strength ratio to express the shear strength estimated from inverse-analyses of field  
102 liquefaction case histories because it was observed that greater pre-failure effective vertical  
103 stress yielded greater liquefied shear strengths. After about two decades, the advantages of  
104 expressing liquefied shear strength as a normalized strength ratio have been recognized (e.g.  
105 Duncan & Wright, 2005; Idriss & Boulanger, 2008).

106 We agree with Dr. Iverson that liquefied strength results from a complex set of micro-  
107 mechanical interactions, however, we do not believe that the strength derived from our  
108 inverse analyses is a fixed material property. We have used a well-known empirical  
109 relationship to parameterize basal shear strength, and have instead derived ‘bulk’ or  
110 ‘apparent’ properties based on an inverse analysis of the distal runout behaviour. We believe  
111 this to be an appropriate simplification due to the ground profile encountered at the site. This

112 point is discussed in more detail below. In the discussion section of Aaron et al. (2017) we  
113 acknowledge that we have not provided a mechanistic description of the strength loss process.  
114 We qualitatively suggest some possible mechanisms that could lead to the residual strengths  
115 estimated based on our inverse analysis.

116

## 117 [Comments from Aaron et al. \(2017\)](#)

118 Dr. Iverson has rebutted a number of comments made by Aaron et al. (2017) regarding the  
119 models of Iverson et al. (2015) and Iverson & George (2016). Here we comment briefly on  
120 this rebuttal.

121 In Aaron et al. (2017) we state that Iverson & George (2016) assumed that the entire volume  
122 of the Oso landslide liquefied. Dr. Iverson rightly points out that his models do not assume  
123 the entire volume liquefies, although they do assume that liquefaction could potentially occur  
124 everywhere. Iverson & George (2016) then predicted the extent of liquefaction based on their  
125 input material properties (discussed below) and “... by gradually increasing the basal pore-  
126 water pressure everywhere within the slope...” (Iverson et al. 2015 p. 204). For the set of  
127 parameters that they claim “...provides a good match to the Oso landslide’s inferred speed  
128 and area of inundation...” (Iverson & George, 2016 p. 181) they predict widespread  
129 liquefaction, which may be less than the entire volume.

130 Our site investigation and modelling results suggest that the models of Iverson et al. (2015)  
131 and Iverson & George (2016) dramatically overpredict the extent of liquefaction. Our  
132 hypothesis, supported by our field and modelling results, is that the large undrained strength  
133 loss was confined to the water-filled colluvium derived from previous landslides along the  
134 slope, and not the unsaturated and overconsolidated slope materials, which did not liquefy.  
135 The models presented by Iverson et al. (2015) and Iverson & George (2016) that best

136 reproduce the impact area of the flowslide predict that the intact slope materials liquefied, a  
137 prediction we find unlikely based on field evidence. This field evidence includes the  
138 overconsolidated and unsaturated nature of the intact slope materials, as well as the thick  
139 accumulation of relatively intact debris deposited in the source zone (Keaton et al., 2014;  
140 Wartman et al., 2016; Aaron et al., 2017; Stark et al., 2017).

141 Dr. Iverson states that “Figure 7 of Iverson and George (2016) illustrates clearly that their  
142 alternative simulations of the Oso landslide each predicted that some landslide material would  
143 be deposited in the source zone, and that the distal deposits of the landslide would be much  
144 thinner than the 30 m claimed by Aaron et al. (2017).” (Iverson, 2018 p.8). While this is true,  
145 no simulations presented by Iverson & George (2016) *simultaneously* predict significant  
146 deposition in the source zone and the geometry of the distal deposits. The observed thickness  
147 of the slump block in the source zone is on the order of 50 m (Keaton et al., 2014; Wartman et  
148 al., 2016; Aaron et al., 2017; Stark et al., 2017). On Figure 7 of Iverson & George (2016), the  
149 only simulation that predicts deposition this thick uses an initial density equal to the critical  
150 state density. For this simulation, the distal deposits are not reproduced. As discussed in  
151 Aaron et al. (2017), the significant volume of material deposited in the source zone is likely  
152 due to brittle failure of the unsaturated and overconsolidated slope material, which did not  
153 liquefy or undergo an undrained strength loss. These intact materials remained frictional and  
154 stayed on the slope (Stark et al., 2017).

155

## 156 Comparison of the Two Dynamic Models

157 Based on the discussion above, we think it is relevant to compare the dynamic analysis  
158 performed by Iverson et al. (2015) and Iverson & George (2016) to that performed by Aaron  
159 et al. (2017). To do this, we first briefly review available site investigation data and then  
160 discuss the modelling objectives and results of the two analyses.

161 Numerous site investigations have revealed that the rupture surface of the Oso flowslide  
162 passed through an overconsolidated, varved, anisotropic glaciolacustrine silt and clay with  
163 infrequent fine sand laminae (e.g. Keaton et al. (2014) Section 5.1; Wartman et al. (2016)  
164 Figure 2; Stark et al. (2017) Figure 2; Aaron et al. (2017) Figure 1). A photo of this unit is  
165 shown in Figure 2, which shows the unsaturated and stiff nature of the glaciolacustrine clay.  
166 The water-filled, loose colluvium highlighted as susceptible to a large undrained strength loss  
167 by many investigators (Keaton et al., 2014; Iverson et al., 2015; Iverson & George, 2016;  
168 Wartman et al., 2016; Aaron et al., 2017; Stark et al., 2017) is derived from this  
169 glaciolacustrine unit. Sand boils were observed in the deposit (Iverson et al., 2015), however  
170 given the absence of a sandy unit in the source zone, we think these are likely composed of  
171 alluvium entrained during motion.



172

173 *Figure 2: Unsaturated and overconsolidated glaciolacustrine silt and clay unit from*  
174 *which the pre-existing colluvium was derived. Photo: J. Aaron.*

175

176 The purpose of the Aaron et al. (2017) runout analysis was to test the hypothesis that the  
177 colluvium underwent a large undrained strength loss/liquefied, whereas the intact slope  
178 materials did not. Aaron et al. (2017) parameterized what they interpreted to be colluvium



179 with strengths representative of a liquefied material (after the strength along the rupture  
180 surface had been reduced to its residual value), and the intact material with strengths  
181 representative of a frictional material with moderate pore-water pressures (i.e, this material is  
182 simulated to be moving in a drained condition). Aaron et al. (2017) then estimated the  
183 strength parameters that best reproduced the observed deposit through an inverse runout  
184 analysis. If Aaron et al. (2017) could reproduce the deposit with liquefied strengths  
185 comparable to other liquefied case histories, and drained strengths similar to those measured  
186 by Stark et al. (2017), then their model results would support the hypothesis that only the  
187 water-filled/saturated colluvium liquefied. This analysis has some discriminatory power,  
188 because if Aaron et al. (2017) had parameterized the entire failed mass with a strength typical  
189 of liquefied material, then they would not have been able to reproduce the deposit  
190 distribution, regardless of the chosen parameters. To meet these objectives, the analysis  
191 presented by Aaron et al. (2017) used a realistic rupture surface (see Stark et al., 2017), and  
192 carefully considered the site stratigraphy and observed distribution of the slide deposits.

193 Iverson et al. (2015) states that “A crucial question regarding the Oso DAF concerns whether  
194 prevailing conditions at the site made landslide liquefaction and high mobility nearly  
195 inevitable ...”, and that “...the question can be addressed in a mechanistic way by performing  
196 alternative dynamic simulations of the landslide. Our simulations are not intended to recreate  
197 the precise details of the slope-failure process at Oso...”(Iverson et al., 2015 p. 204). Based  
198 on this, we think the purpose of the dynamic simulations presented in Iverson et al. (2015)  
199 and Iverson & George (2016) was not to provide a detailed inverse analysis of the event,  
200 instead they focus on inferences into the landslide dynamics that can be gained from their  
201 simulations. When interpreting their simulation results for this purpose, we think two  
202 important limitations must be acknowledged: (1) Iverson et al. (2015) and Iverson & George

203 (2016) did not use a realistic rupture surface and (2) the material properties used by Iverson et  
204 al. (2015) and Iverson & George (2016) do not represent the site stratigraphy.

205 The dynamic analysis presented in Aaron et al. (2017) uses a compound rupture surface above  
206 the water-filled colluvium, which is appropriate for the anisotropic glaciolacustrine unit (Stark  
207 et al., 2017) and supported by numerous surface and subsurface observations (Aaron et al.,  
208 2017; Stark et al., 2017). The dynamic analyses presented by Iverson et al. (2015) and  
209 Iverson & George (2016) use a deep-seated rupture surface based on a logarithmic spiral,  
210 which is only relevant for homogenous and isotropic materials (Stark et al., 2017). No  
211 compelling site investigation data exist to support this interpretation of the rupture surface.  
212 This logarithmic spiral rupture surface is also steeper than our compound rupture surface, and  
213 therefore results in a greater imbalance between driving gravitational forces and resisting  
214 forces, once liquefaction has occurred in their model. This, combined with their prediction of  
215 widespread liquefaction, is the likely reason that the models of Iverson & George (2016)  
216 cannot explain both the significant deposition in the source zone and the geometry of the  
217 distal deposits.

218 The material properties used by Iverson et al. (2015) and Iverson & George (2016) "...were  
219 inferred from laboratory testing of sediment mixtures used in landslide and debris-flow  
220 experiments that involved uncompacted materials similar to the predominantly sandy material  
221 observed at Oso (Iverson et al., 2000, 2010)." (Iverson & George 2016, p.181). As  
222 summarized above, the Oso flowslide was not composed of a predominantly sandy material.  
223 It is not obvious to us that an accumulation of water-filled/saturated, fine grained colluvium  
224 along the slope toe derived from the grey glaciolacustrine silt and clay would behave like a  
225 sandy material similar to that experimentally tested by Iverson et al. (2000 and 2010). As  
226 discussed in Aaron et al. (2017), the colluvial material at Oso is more likely composed of

227 disaggregated blocks of overconsolidated glaciolacustrine silt and clay with water filling the  
228 cracks and fissures between the blocks.

229 Given the discussion above, we think there are significant limitations to the simulations  
230 presented by Iverson et al. (2015) and Iverson & George (2016). However, with appropriate  
231 use of judgement, these models can provide important insights. They support the hypothesis  
232 that a large undrained strength loss occurred at the site (although, as summarized above, they  
233 likely overpredict the extent of liquefaction), elucidate the process of pore-pressure  
234 dissipation at the margin of flowslides, and demonstrate the sensitivity of undrained strength  
235 to initial porosity. In interpreting these results, however, it is important to realize that details  
236 of the site stratigraphy were not known at the time Iverson's simulations were performed, so  
237 the physical relevance of the parameters used (if interpreted as true material properties)  
238 requires further justification. The concepts of in-situ and critical state density for a  
239 heterogeneous, overconsolidated, and anisotropic glaciolacustrine silt and clay unit need to be  
240 meaningfully defined. The dynamic model used by Iverson et al. (2015) and Iverson &  
241 George (2016) can only simulate the liquefaction of a loose granular material, a limitation that  
242 must be disclosed and does not represent the majority of the soils involved in the slide.

243

## 244 **Summary**

245 We believe that a meaningful geotechnical analysis must balance site characterization and use  
246 of idealized models, all moderated with judgement. With this in mind, numerical dynamic  
247 models should be used within a complete analysis framework that carefully considers the site  
248 stratigraphy. This is how we set-up and interpreted our dynamic modelling results. The shear  
249 strengths we have derived from inverse analyses correspond to an apparent residual strength  
250 attained by the water-filled colluvium along the slope toe during the runout phase of motion.

251 The inverse analysis results, based on a geotechnically feasible rupture surface and a detailed  
252 interpretation of the site stratigraphy and the landslide deposits, support the hypothesis that  
253 two distinct mechanisms occurred during the Oso flowslide (Keaton et al., 2014; Wartman et  
254 al., 2016; Aaron et al., 2017; Stark et al., 2017). One mechanism is a large undrained strength  
255 loss of the water-filled/saturated colluvium that mantled the slope toe due the impact of a slide  
256 mass further upslope. This colluvium was derived from an overconsolidated, varved,  
257 glaciolacustrine silt and clay layer. The second mechanism is brittle failure of unsaturated,  
258 overconsolidated, and non-liquefiable intact slope material including glacial till and dense  
259 outwash sands. Our numerical modelling results do not provide a mechanistic description of  
260 the large undrained strength loss of the water-filled/saturated, fine grained colluvium.  
261 However, we do suggest some potential mechanisms that could have resulted in the observed  
262 undrained strength loss of the colluvium. Further research into this topic is warranted.

263 Our analysis equally weighs consideration of the ground profile and results from idealized  
264 models. We have made careful use of empiricism to support our conclusions, while  
265 acknowledging the limitations of our analysis. Due to this, we think we have identified one of  
266 the key unanswered questions regarding the Oso flowslide: How can an accumulation of  
267 colluvium, derived from overconsolidated, varved, glaciolacustrine silt and clay, undergo a  
268 large undrained strength loss/liquefy? This is not a material conventionally recognized as  
269 liquefiable, and it is not obvious how the mechanics of liquefaction of loose, saturated sands  
270 apply to this material as Iverson claims. As this conclusion acknowledges the complexity of  
271 the site stratigraphy, it is useful for researchers and practitioners trying to predict the future  
272 behavior of similar slopes.

273

## 274 Acknowledgements

275 This closure was greatly improved through many insightful discussions with Dr. Scott  
276 McDougall.

277 We were greatly saddened by the passing of Oldrich Hungr last year. He was an insightful  
278 researcher, a patient advisor, and a great mentor. He approached scientific debate with  
279 respect, an open mind, and good humour, and we agree he would have welcomed this  
280 discussion. He was an integral part of this work, and his insight into the present discussion is  
281 greatly missed.

## 282 References

- 283 Aaron, J., & Hungr, O. (2016). Dynamic analysis of an extraordinarily mobile rock avalanche  
284 in the Northwest Territories, Canada. *Canadian Geotechnical Journal*, 53(6), 899–908.  
285 <https://doi.org/10.1139/cgj-2015-0371>
- 286 Aaron, J., Hungr, O., Stark, T. D., & Baghdady, A. K. (2017). Oso, Washington, Landslide of  
287 March 22, 2014: Dynamic Analysis, 143(9). [https://doi.org/10.1061/\(ASCE\)GT.1943-](https://doi.org/10.1061/(ASCE)GT.1943-5606.0001748)  
288 5606.0001748.
- 289 Burland, J. B. (1987). The Teaching of Soil Mechanics - A Personal View. Nash Lecture. In  
290 *Proc. IX European Conference on Soil Mechanics and Foundation Engineering, Dublin*  
291 (pp. 1427–1447).
- 292 Duncan, J. M., & Wright, S. G. (2005). *Soil strength and slope stability*. John Wiley & Sons.
- 293 Goodman, R. (1999). *Karl Terzaghi The Engineer an Artist*. Virginia: ASCE Press.
- 294 Hungr, O. (1995). A model for the runout analysis of rapid flow slides, debris flows and  
295 avalanches. *Canadian Geotechnical Journal*, 32, 610–623.

- 296 Hungr, O. (2016). *A review of landslide hazard and risk assessment methodology*. (S. Aversa,  
297 L. Cascini, L. Picarelli, & C. Scavia, Eds.), *Landslides and Engineered Slopes.*  
298 *Experience, Theory and Practice: Proceedings of the 12th International Symposium on*  
299 *Landslides (Napoli, Italy, 12-19 June 2016)*. CRC press. [https://doi.org/10.1201/b21520-](https://doi.org/10.1201/b21520-3)  
300 3
- 301 Hungr, O. (2017). Chapter 9 - Runout Analysis. In M. Hawley & J. Cunnig (Eds.),  
302 *Guidelines for Mine Waste Dump and Stockpile Design*. CSIRO Publishing.
- 303 Hungr, O., Dawson, R., Kent, A., Campbell, D., & Morgenstern, N. R. (2002). Rapid flow  
304 slides of coal-mine waste in British Columbia, Canada. *Geological Society of America*  
305 *Reviews in Engineering Geology*, 15, 1–18.
- 306 Hungr, O., & Evans, S. G. (2004). Entrainment of debris in rock avalanches: An analysis of a  
307 long run-out mechanism. *Geological Society of America Bulletin*, 116(9), 1240–1252.  
308 <https://doi.org/10.1130/B25362.1>
- 309 Hungr, O., Leroueil, S., & Picarelli, L. (2014). The Varnes classification of landslide types, an  
310 update. *Landslides*, 11(2), 167–194. <https://doi.org/10.1007/s10346-013-0436-y>
- 311 Idriss, I. M., & Boulanger, R. W. (2008). *Soil Liquefaction during Earthquakes*. Engineering  
312 Monographs on Earthquake Criteria, Structural Design, and Strong Motion Records,  
313 Monograph #12, MON-12, Earthquake Engineering Research Institute, EERI, 499 14th  
314 Street, Suite 320, Oakland, CA 94612,.
- 315 Iverson, R., & George, D. (2016). Modelling landslide liquefaction, mobility bifurcation and  
316 the dynamics of the 2014 Oso disaster. *Géotechnique*, 66(3), 175–187.  
317 <https://doi.org/10.1680/jgeot.15.LM.004>
- 318 Iverson, R. M. (2018). Discussion of Case Study: Oso, Washington Landslide of March 22,

319 2014: 4 Dynamic Analysis" by J. Aaron et al. *ASCE Journal of Geotechnical and*  
320 *Geoenvironmental Engineering*. [https://doi.org/doi: 10.1061/\(ASCE\)GT.1943-](https://doi.org/doi: 10.1061/(ASCE)GT.1943-)  
321 5606.0001748

322 Iverson, R. M., George, D. L., Allstadt, K., Reid, M. E., Collins, B. D., Vallance, J. W., ...  
323 Bower, J. B. (2015). Landslide mobility and hazards: implications of the 2014 Oso  
324 disaster. *Earth and Planetary Science Letters*, 412, 197–208.  
325 <https://doi.org/10.1016/j.epsl.2014.12.020>

326 Iverson, R. M., Logan, M., LaHusen, R. G., & Berti, M. (2010). The perfect debris flow?  
327 Aggregated results from 28 large-scale experiments. *Journal of Geophysical Research*,  
328 115(F3), F03005. <https://doi.org/10.1029/2009JF001514>

329 Iverson, R. M., Reid, M. ., Iverson, N. ., LaHusen, R. ., Logan, M., Mann, J. ., & Brien, D. .  
330 (2000). Acute Sensitivity of Landslide Rates to Initial Soil Porosity. *Science*, 290(5491),  
331 513–516. <https://doi.org/10.1126/science.290.5491.513>

332 Keaton, J. R., Wartman, J., Anderson, S., Benoit, J., DeLaChapelle, J., Gilbert, R., &  
333 Montgomery, D. R. (2014). *The 22 March 2014 Oso Landslide , Snohomish County,*  
334 *Washington*. Geotechnical Extreme Event Reconnaissance Association Report GEER-  
335 036. <https://doi.org/10.18118/G6V884>

336 McDougall, S., Boulton, N., Hungr, O., Stead, D., & Schwab, J. W. (2006). The Zymoetz  
337 River landslide, British Columbia, Canada: description and dynamic analysis of a rock  
338 slide–debris flow. *Landslides*, 3(3), 195–204. <https://doi.org/10.1007/s10346-006-0042-3>

339 McDougall, S., & Hungr, O. (2004). A model for the analysis of rapid landslide motion across  
340 three-dimensional terrain. *Canadian Geotechnical Journal*, 41, 1084–1097. Retrieved  
341 from <http://www.nrcresearchpress.com/doi/abs/10.1139/t04-052>

342 Olson, S. M., & Stark, T. D. (2002). Liquefied strength ratio from liquefaction flow failure  
343 case histories. *Canadian Geotechnical Journal*, 39, 629–647.  
344 <https://doi.org/10.1139/T02-001>

345 Sosio, R., Crosta, G. B., & Hungr, O. (2008). Complete dynamic modeling calibration for the  
346 Thurwieser rock avalanche (Italian Central Alps). *Engineering Geology*, 100(1–2), 11–  
347 26. <https://doi.org/10.1016/j.enggeo.2008.02.012>

348 Stark, T. D., Baghdady, A. K., Hungr, O., & Aaron, J. (2017). Case Study : Oso , Washington  
349 , Landslide of March 22 , 2014 — Material Properties and Failure Mechanism, 143(5).  
350 [https://doi.org/10.1061/\(ASCE\)GT.1943-5606.0001615](https://doi.org/10.1061/(ASCE)GT.1943-5606.0001615).

351 Stark, T. D., & Mesri, G. (1992). Undrained Shear Strength of Liquefied Sands for Stability  
352 Analysis. *Journal of Geotechnical Engineering*, 118(11), 1727–1747.  
353 [https://doi.org/10.1061/\(ASCE\)0733-9410\(1992\)118:11\(1727\)](https://doi.org/10.1061/(ASCE)0733-9410(1992)118:11(1727))

354 Wartman, J., Montgomery, D. R., Anderson, S. A., Keaton, J. R., Benoit, J., deLa Chapelle,  
355 J., & Gilbert, R. (2016). The 22 March 2014 Oso landslide, Washington, USA.  
356 *Geomorphology*, 253, 275–288. <https://doi.org/10.1016/j.geomorph.2015.10.022>

357